

FED.ROAD STATE PROJ. HO. TEAR HO. SHEETS

1 WASH. WPMS-269-A 1936 12 16

-		BAR	LIST	,	A11 Dim	ensions Out. to Out. Angles = 45°		•
Mark	Loc	ation		Size	Length	Bending Diagram	Mark	
1	Handrail 1	Top H to J	2	3/4 Φ	39-4		48	Sidew
2	M	• D to H	4	3/4 ¢	37-2"		49	Curb
3	41	4 J to 0	4		32-9		5`0	
4		do	2	 -	32-6	35'-8"	5 I 5 2	
5	*	O to E. e			35-10		53	
6	•	W. end to	D 4		32-10		54	Sidew
8	n D	do nel X Bottom H to J	4		32'-8" 39'-4"		55	Sidew
9	" Fai	" Dto H			36-8		56	
10	**	" J to O			32'-4"		57	
11	•	al 68			32-2"	Straight		
12		- Oto E. e	nd 8		35′-5″		58	
13		" Wend to			32-5		59	W. W. F
14		do	4	1/2 4	32'-3	5traight 1'-2"+3	60	
15	3 e	Main Posts Ver	+. 5 C	3/4 4	11- 2	<u> </u>	61	
				-/4"	, ,	5'-3" ————————————————————————————————————		
16		* @ Exp. Jt	s. 4	74 4	6 '- 7"		62	
17	•	4 Hoops	. 15	1/2 4	5-6		63	
				7			64	
18	• }	Panel Posts Ver		4 4	 	5-0	65	<u> </u>
19		do	36	44	5-8	·N 1'-3 → 3' - 9' - 1 2 N		:
20	•	Ver	t. 129	1/2 4	4'-0"	<u>w[]w</u>	66	
ł					ļ	3'-0"	67	
21	Coping	H to J	1		37-8		68	
-22	*	D to H	2		37-9		69	
23	9	J +0 O	3 nd 2		32'-4" 36'-0"		70	
24	н	O to E.e W. end to		-			70	
26	Girder ı	under rail H+oJ			38-9	37' 0"	71	
27	11	DtoH	8		38-3	25. 0"	72	
28		J +0 O	8		l		73	
29		• •	4	3/4 4	32'-6	Straight	74	
30	gê.	0 to E. e	nd 8		37-0		75	
31	40	W. end to					76	
32	e4		4	1'4	33'-0"	1 0' 0" 11'E" 11' 0" d		
33	*	• •	2	1" +	37-6		77	
34	k	te	2	l' ¢	39-0	13'-0" - 11'-0" - 11'-0" - 1	78	
					 		79	
35	•	D to H	2	3/4 4	43'-0"	1-8-10-10-11-5-7-8-1-5-7-10-1-5-7-10-	80	
36		H to J	1	3/4 4	42-0	1-4-4-41-5-7-10-41-5-7-8-41-5-7-10-41-5-4-4-4-4		
		ALC:		- 			81	Floor
37	•	J +0 O	2	3/4 4	43'-0	1(-8" 10-10" 1-5 -7-8" 1-5 -7'-10" -1-5 -7-10"		
38		*		3/4 4	27-2	7'-8"-11'5"-7'-6"-	82	**
							83	
39	*	Oto E.e.	nd I	3/4 9	43'-0"	1-5-7-10-1-5-7-10-1-5-7-10-1-5	84	
40				3/4 4	36-0		9.5	
				_		7-10-1-5-1-1-5-1-1-5-1-10-0"	85	
41	*	@ Posts	2.4	. 3/4° d	9-2		86	
						- 2'-0"+ 2'-1		
42	30	@ Exp. Jts	s. ව	3/4" 4	7-6	- 1'-7"-+-2-4+	87	Hoops
43	**	Hoops	200	9 1/2" d	4-6"	h 1'-6"	88	
	,						89	
44	Sidewalk		8		37-6		90	
45		D to H			37'-0		92	Hoops
46	•	" J+00			32-8		93	- cops
47	h	" W. end to	עו 24	78	Varies	. 16@32-8 8 (32-8 + 28-8)	33	<u></u>

			r 1	T	
Mark	Location	No.	Size	Length	Bending Diagram
48	Sidewalk Longit. Oto E. end	16	3/8 4	36-2	Straight
49	Curb Beam H to J	4	³ / ₄ φ	· · · · · · · · · · · · · · · · · · ·	do
5'0	" D+0H	_8_	_	38-0	d o
51	" J to O	12	3/4 ¢		
52	W. end to D * E. end to O	8	3/4 4		do
5 3		ļ	3/8 ¢		6'-5"
54	Sidewalk Transv.		ļ		<u> </u>
55	" " W. end	8	+	Varies 7'-3"	do Ave. 3'-9"
56 57	· · W. end	220 8	ļ	Varies	do Ave. 3'-6'
			 		_ - 1-7-43 - 2-6 43 1-10-44].
58		440			
59	W.W.P. Girder W. end + D	12	۱ ф	31-6	30'-0"
60	16 16	4	i o	32-6	15' 9" 14' - 3"
	t	2	1" +	44-9	العراق المراجع العراق من العراق المراجع العراق العر
61	<u> </u>	2	1 4	44.9	<u>×</u>
62	" D+• H	16	1" 4	41-6	
63	м н	2	l" ф	4 3-3"	2'-4' - 8'-6" - 2'3' - 8'-0" - 2'3' - 6'-0" - 2'3' - 6'-0" - 2'3' - 6'-0" - 8'
64	*	2	i"do	4 3′-6	2-4-10-7= 2-3-6-0-2-3-8-0-6-0-
			 		35' - 0"
65	• H+oJ	8	 	38-0	2'9 - 8'-6'-12'3 - 6'-0"-12'3 - 6'-6'-12'9
66	*	2	IΦ	40-8	
67	- Jto O	16	1" +	34-7	33'-1"
68		8	1"+	33-1	
69		4	1" 4	43-6	2.6 - 10 7 2 2 3 - 6 - 0 - 2 3 - 6 - 0 - 2 3 - 6 - 0 - 2
					p-6:0°-12:3]- 8'-0" - 12:3]- 6'-0"-1
70	F	2	1 4	26-4	<u> </u>
71	• Oto E.end	16	1"4	27-6	
72	g H	2.	1"+	44-8	2'.9" - 9'-6" 2'.3 - 7'-0" - 2'.3 - 8'-0" - 2'.3 - 6'-0"
73	• @ E,I,M,P	8	3/4° φ	12'-0	
74	" Hoops Wend to B	_	 	Varies	
75	# #		+		
	. Usana S.A. F. and			10'-4"	7 12:21
76	· Hoops B to E. end		 		
77	gs 64 14	204	 -	10'-0	12-61
78	· @ Exp. Jts.	4	ľΦ	12'-0"	Straight
79		8	Ι" Φ	14-6	2'-0' 2'-0'
0.0	* " Uaane	E 2	1/2 4	6'-2"	'-Q"
80	" - " Hoops	52			b 0' 6" 45'2" 5'-3'-12'6"
81	Floor Beams C,E,F,G,I,K,L,M,N,P,Q	30	1 4	24-3	
	D, H, J, O				9'- 9"
82	" E,F,G,I,K,L,M,N,P,Q,D,H,J,O	14	1 4	24-9	
83	do	28	l" o	25'-4"	22-4-
84	d₀	28	+	25'-6"	10' 4" 3" 3"
			l' ¢		6,-6,-41,10,-
8.5	D. H, J, O	4	P	10.0	
86	do	4	l o	15-3	6'-9" 7'-0"
			1.4		
87	Hoops C,E,F,G,I,K,L,M,N,P,Q,B	 	½ ¢		
88	" " D,H,J,O @ back	16	1/2 4		
89	do	16	1/2° Φ		
90	do	16	1/2 ¢		
			1/2 4	-	
92	Hoops E,F,G,I,K,L,M,N,P,Q,D,H,J,O front		/ ₂ Φ		
93	do	14	/2 4		93

	1	**			
Mark	Location	No.	Size	Length	Bending Diagram
94	Hoops E,F,G,I,K,L,M,N,P,O,D,H,J,O front	 	½° 4		
95	do	14	1/2 +	8.8	+ t t
96	E, F, G, I, K, L, M, N, P, Q	10	/ ₂ ф		
97	do DULO L	10	1/2" 4 1/2" 4	9'-6'	
98	Hoops D,H,JO back to front	16	/2 4		*
100	do	12	1/2 4	_	
101	do	16	1/2 4		4 (976) (359) (199 (191) 4(17) (143)
102	do do	8	/2 [*] φ /⁄2 [*] φ		
104	do	4	1/2" 4		1 (A) TI
105	Beam B Longit.	2	ı°φ		
106	do	2	1" 4		15' - 6'
107	do	3	{" o	19'-0"	
108	Old Caps	68	j's	4'-9"	3-3-
109	Hoops	17	3/44	6'-2"	1
110	u w	17	3/40	-	
111		17	3∕4 ♦		
112	• 4	17	3/4*		OP' A"
113	Beam C	2	†···	27'-6"	
114		1	1"4	27'-0	
115	N ;	2	l" ♦	28-0	16'-8" 10'-0"
116	" Hoops front	7	1/2 4	Varies	Ave. L. 9'-0"
117	do	4	1/2 4	Varies	Ave. L. 6'-8"
811	do	1	1/2 4	4'-10"	<u>9</u> L <u>1'-2"</u>
119	Beam @ sect. D-D	2	140	36'-0"	34′-6″ —
120	do	2)" B	3 ළ ′-0්	16-6" - 12-3 - 9'-0" + 8'-0"
121	do	3	140	24-3"	
122	do	1	14"0	27'-3'	
123	do	1	140	21'-6"	
124	dò	1	140	24'-0"	4'-6 - 11' - 0" - 3'-9 4'-6 - 14' - 0" - 3'-9
125	do	1	140	26-6	17'-0"—+ £5"
126	do	ı	+	28-6	CITY STREET
127	" Hoops back to front				WIDENING POST STREET BRIDGI
128	de	4		12'-4"	OVER SPOKANE RIVER
129	· do	2		12'-8"	SPORANE COUNTY
130	d o	2		13-0	Av. L. 13'-6" 4'-10" DEPARTMENT OF HIGHWAYS OLYMPIA, WASHINGTON
131	do do	6	+		AV. L. 13-6 4-10 APPROVED: MAY 21, 1936
		↓ ·			AV. L. 12-1 4-4 to BRIDGE ENGINEER DIRECTOR OF HIGHWAYS
133	<u>.</u> . do	.		 	SHEET) OF A SHEETS
					OSMINACI NOMBER
135	do Beam Sect. E-E	2		Varies 40-0	DEPT OF PUBLIC WORKS - ENGINEERING DIVIS
137	do do	2		37'-2"	- 7'-6" - 3'-0" - 15' - 0" - 3'-0" - 8'-6" - 8'-6" WIDENING
138	do -	2		38'-6	BAR LIST
	•	 		 	BJGARNETT, ENGINEER
139	do	2		37-0	APPROVED.
140	do Hoops	28	1/2 4	10'-0"	9
-	7-13-36 Changed Bars Nos.1. DATE REV	5,16,18, 1 S 1 C		3/4."\$	DI.ME BY CHKD

C-18 624 8/303

BAR LIST (CONT.)

r 1	DAN E					
Mark	Location		No.	Size	Length	Bending Diagram
141	Beam @ E-E (cont.)	loops front	2	1/2 4	Varies	Ave.L. 9-6"
143		-	2	ار" ه	Varies	2'-10' k 2'-7' Ave. L. 8'-5"
142	do		2		Varies	2'7 2'-3"% 1'-10" Ave. L
144	Beam @ sect. A-A	*	4	1 7	40-6	39'-0"
145	do		4	ι" ф	39-0	Straight .
146	do		2	ı" φ	49-6	
						-4-6-2:3-13-0'
147	do		2	ſФ	39-3	
			,	(* ¢		9'-0"-12'3
148	do		1	, ,	333	
149	do		2	1. 4	38-0	<u>14-0</u> 14-0 12-1 12-1 12-1 12-1 12-1 12-1 12-1 12
150	do	Ноорз	82	1/2 4	7-8	2'-7'
		_				24-0
151	Beam @ sect. B	-B	4	 	25.6	
152	do		4	1" 4	24'-0"	Straight4'-0"-+2'9 6'-0"-+2'9 6'-0"-+
153	· do		2	1" ф	25-6	4'-0"-12'9 - 6'-0"-12'9 - 6'-0"-1
					5	10'-0"-12'0 9'-0"
154	do		2	ј* ф	37-0	2
155	do		2.	ј" ф	39-0	13'-0" 12'-0" 10
						107
∤56	do	Hoops	51	½ ¢	8 '-2"	2'-10"
157	Barm @ M.		4	1" 4	17'-0	Straight
157	Beam @ M.H.	w. http://www.	~			
158	Floor Slab Longit	t. H toJ	15	1/2° Φ	37-o	Straight
159	do	D to H	30	1	37-4	do
160	do	J to O	45	1/2 4	31-9	do
161	do	0 to E, end	30	1/2 4	Varies	Ave. L. 30'-0"
162	do	D to B	15	1/2 4	37 ⁻ 0	Straight
163	· do	BtoW.and	30	1/2" Ф	Varies	Ave. L. 29-0"
164	do		19	1/2" ф	Varies	Ave. L. 35 ² 0
165	Floor Slab Tran	sv. O to E. end	350	1/2 ¢	16-10	16'-6"
166	do		175	1/5" =	18'-0"	- 2'-3"- 5" 7'-6"- 5 - 1'-9" 5 - 4'-0"
<u> </u>			ļ		Varies	- 2'-3"-15 - 7'-6"-15+1-5+5-4-0"+ Ave. L. 22'-0"
167	do	B to C	-	 	Varies	AVE.L. 22'0" AVE.L. 22'0"
168 169	do do	W. end to B	33	 _ _	Varies	Ave.L 21'-0"
169	QO	W. End to D	 	 		
170	do		<u> </u>	ļ	Varies	15 S S S Ave. L. 24'-0"
170	do	, ,	 	5/8 Φ	l	40. 0.
172	Beam over arch	B to D	4	Į.	41-6	
173	do	D to H		—	39.0	
174	do	H to J	 	 	39-0	32'- 6"
175	do	J + 0	12	3/4 4		3
176	do	O to E. end	4	3/4 Φ	 	31' - 0"
177	do		4	3/4 · Φ	<u> </u>	Straight -4'-0"-12:6-8'-0"-12:6-8'-0"-12:6-2'-6-4
178	do	B+o D	2	³ / ₄ Φ	42'-0"	
179	do	D+oH	2	1 ф	27-0	2:9 -9-0:+ 2:9 -10-0:-
180	do	<u>, , , , , , , , , , , , , , , , , , , </u>	2	1" ф	27-0	<u>► 2'-95 2:5 + 7'-6" 2'5 +-10'-0"-+ </u>
181	do		2	Ι" φ		
01	40		,	 		p-2:s/2:d-8:-0-+2:d-8'-0"+2:d-8'-0"+2:d-2'-9"+
182	do	Lot H	2	ј" ф	44-6	
183	do	J +0 O	2.	1" 4	27-0"	2.3 — 90 — 15.3 — 100 — 1
104	<u> </u>		2	ј" ф	2.7'-0	+ 2'-9'+2'-3'7'-6'+2'-5 10-0"-+
184	do		 	 		1-6'-0'-12'-6' 8'-0'-12'-6'6'-0'-12'-68'-0'1
185	do		4	1" ф	38-6	2
186	do	0 to E end	2.	1" 4	35-8	2-5-42-6-8-6-6-6-6-8-8-8
				ļ		
187	do		2	1" Ф	38-6	8'-0"-12'd 8'-0"12'd 5'-0"
188	do	Hoops	213	½" ф	7:4	<u></u>
1			<u> </u>	<u></u>	<u>'l</u>	2'-8"

Mark	L	oca'	tion	No.	Size	Length	Bending Diagram
189	Column	@	Wand & Q	8	ί Φ	21'-0"	Straight
190		do		8	ј" ф	7'-0"	6'-0"
191	Column	6	W.end XQ Hoops	34	1/2 Φ	Varies	Ave.L. 8'-4"
192	Column	©	Ъ	4	ļ* ф	28'-6"	Straight
193		do	D K D	8	1° 4	16'-0"	do
194	Column	•	B Hoops	23	-	Varies	Ave.L. 9'-4"
195	Column	@	С	4	į Φ	25-0°	Straig ht
196	н	e	C, D, E, M, N, O	24	Ι' Φ	7'-0"	. do
197	•	•	C Hoops	20	1/2 4	Varies	Ave. L. 8'-4"
198	Column	e	D	2	l" +	18'-0"	Straight
199		do		2_	1" 4	17-0	
200		do	Hoops	14		Varies	19 N Ave. L. 7'-8"
201	Column	@	E	4	1" 4	 	Straight
202		do	Hoops	8		Varies	Ave. L. 6-10"
203	Column	•	F	4	1" 4	10-6	Straight
204		do	Hoops	4	'∕2 [*] ♥	Varies	Ava. L. 6'-8"
205	Columns	@	G X K	8	1' +	9'-0"	Straight
206		do	Hoops	4	1/2 ♦	6'-4"	
207	Column	. @	н » Ј	4	1" φ	7'-0"	Straight
208		do	- 10.00v	4	1" ф	5′-6″	4-0"-
209	Column	•	1	4	l" ф	6-6	Straight
210	Column		L	4	1" 4	12'-0"	do
211		d o	Hoops	6	1/2" 4	Varies	الله الله الله الله الله الله الله الله
212	Column	•	M	4	1 4	15-0"	Straight
213		do	Hoops	10	½ ቀ	Varies	49 Ave. L. 7-0"
214	Column	@	N Hoops	17	1/2 +	Varies	Ave. L. 7'-10"
215	Column	•	N	4	1, 4	21-0	Straight
216	Column	@	0	2	l" 4	28-6	do
217		do		2	1" 4	27 ⁻ 0	25- 6.
218		do	Нооря	23	1/2 4	Varies	Ave. L. 8'-6"
219	Column	•	Р	4	1" 4	33'-0	
220		do	Hoops	28		Varies	,3.°9, [1,8]
221	Сар Ноор		W.end,C,D,E,F,G,H, K,L, M,N,O,Q	28	½° Ф	8-4	, of 12.00
222	Cap Hoop	s @	В% Р	4	½ ф	10-0	
223	Arch	•	abuts.	56	- 	Varies	34 @ 14'-0"; 22 @ 17'-0"
224	Arch		,	168	 	В	22 @ 47-6; 34 @ 46-0; 56 @ 44-0; 28 @ 41-6; 28 @ 51
225	Arch i	100p	s BtoC & PtoO		-	Varies	6'-8" +0 6'-2"
226	do		CtoD X OtoN	104	1/2" Ф	Varies	
227	de		DtoE X MtoN		1	Varies	A 11
228	de		EtoF X LtoM	+ -		Varies	
229	de		FtoG X KtoL	-+	+	Varies	AVE. L. 14 6
230	do	· · · · · · · · · · · · · · · · · · ·	G+.H & J+oK	96	1/2 9	Varies	Ave L. 14-3

	PED. ROAD S	STATE	PROJ. NO.	FISCAL YEAR	SHEET No.	TOTAL SHEETS
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		 	1 1		
Mark	Location	No.		Length	Bending Diagram
232	Struts	48	1" 4	13′-0″	Straight
233	do Hoops & D&N	20	½* ¢	17'-4"	· · · · · · · · · · · · · · · · · · ·
			1, -		5′- 9″ — l'd
234	do FXL	20	1/2 4	15'-10	5'-0"8
235	do H&J	20	1/2 +	15- 2"	4'-8"
236	Manhole @ W.end Sides	15	1/2 4	32-0	° - 6
					10'- 6"
237	do Footing	3	3∕4 Φ	10,0	7'- 6'-
238	do "	3	3/4 4	8-6	8'~ 0"
239	do •	3	3/4 +	11'-0"	10′- 6″-
240	ර	4	½ ¢	3'-9"	3'-6"
241	do vert. N. Wall	9	½° Ф	16'-6"	Straight
242	do End	3	½" ф	13'-0"	do
243	do S. Wall	8	1/2 Φ	15'-6"	do
244	Manhole @ E.end Sides	21	1/2" +	30'-0"	8'-9"
		-		<u> </u>	11'-3"
* 245	do Footing	3	3/4 Φ		
246	do	3	3/4° Φ		
247	do	3	3/4 Φ		11'- 6"
248	do vert. S. Wall	8	½ ° Ф	22'-6"	
249	do end	3	½ ¢	20-0	
250	do N.Wall	7	1/2 4	23'-0"	do
251	Wall @ E. end vert.	18	1/2" \$	22'-6"	d ಂ
252	do	1	1/2° 4	2 3'-6'	
253	do Horiz.	24	1/2° Ф	19'-0"	18′ - 6″ -
254	do	8	1/2 4	21'-6"	
					2-0-19-4-0-19-4-0-19-4-0
255	do	21	1/2" 4	4'-0"	3'-6"
25 6	Coping @ W. End	8	i" 4	30'-0"	
257	Girder Bars at Exp. Joints	32	3/-6		
258	Arch rib hinges	21	2**	15-0	Str.
259	do	24	2**	17'-6"	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
	1	 			3-4-02 1-6 4-02 3-
260	do	12	2*"	14'-2"	30
			5.00		3-4" 2-7 1-6" 2-7
261	Hinge stirrups	10	300		2,-8, 3, 4, 3, 4, 3, 4, 3, 4, 3, 4, 4, 3, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4, 4,
262	do	5	₹"	22'-0"	2,-8, 7, 9, 4, 6,
				1.	ाW हिं.विका
263	do	84	15.0	(Ave.)	4 Varie
200					1-9", 2
			, uh	(Ave)	Variable diam. to
264	Hinge spirals	18	左"中	(99'-0")	wrap bars 259 & 260.
					flor
	,				
<u> </u>	ļ				
					•

8-20-36 Bars 223, 224, 231, & 258 to 264 A.M.B. W.T.R.
7-13-36 Added Bar No. 257 DI.M.S.
DATE REVISION BY CHKD.

CITY STREET CITY OF SPOKANE IDENING POST STREET BRIDGE OVER SPOKANE RIVER SPOKANE COUNTY STATE OF WASHINGTON

DEPARTMENT OF HIGHWAYS OLYMPIA, WASHINGTON . APPROVED: MAY 21, 1956

BRIDGE ENGINEER

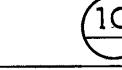
SHEET 12 OF 14 SHEETS

CITY OF SPOKANE
DEPT. OF PUBLIC WORKS ENGINEERING DIVISION

POST STREET BRIDGE

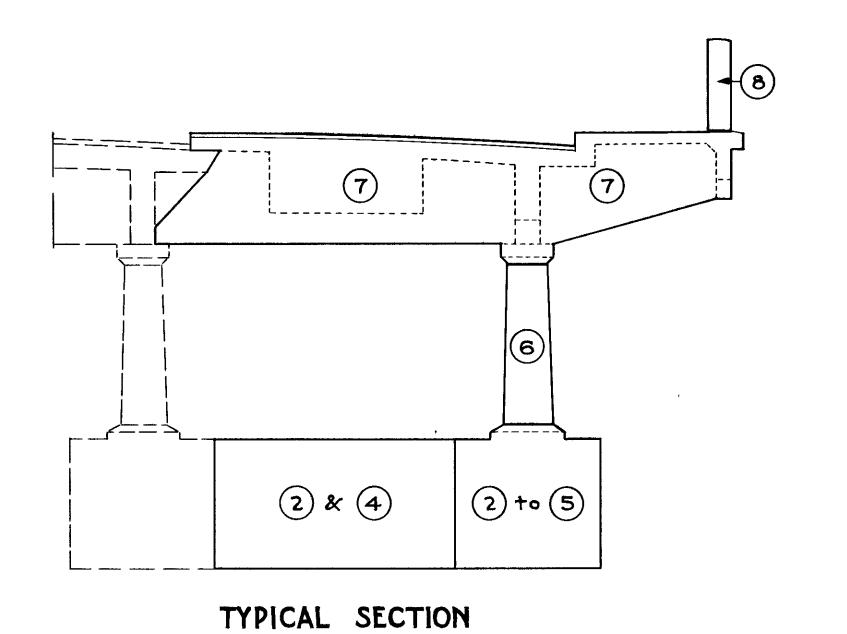
WIDENING BAR LIST

NOVEMBER 1935 B. J. GARNETT, ENGINEER



C-18 624

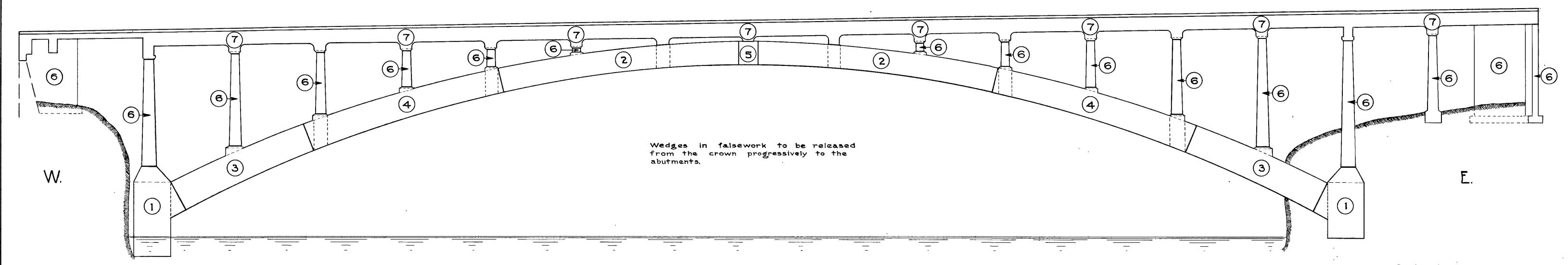
8/304



DATA DESIGN H-15 Loading

CLASS "A" CONCRETE fc = 1000 #/u" CLASS "B" CONCRETE fc = 750 #/a" fs = 18000 #/p"

	CONCRETE PLACIN	NG
Order	Location	Number
1	Abutments	1
2	Arch & Struts	2
3	Arch	3
4	Arch & Struts	4
5	Arch	5
6	Columns, Walls & M.H.	6
7	Roadway	7
8	Handrail	8



SECTION ON & OF NEW ARCH

CITY STREET CITY OF SPOKANE WIDENING POST STREET BRIDGE OVER SPOKANE RIVER SPOKANE COUNTY

STATE OF WASHINGTON DEPARTMENT OF HIGHWAYS OLYMPIA, WASHINGTON

APPROVED: MAY 21, 1936

SHEET 13 OF 14 SHEETS CITY OF SPOKANE

DEPT OF PUBLIC WORKS-ENGINEERING DIVISION

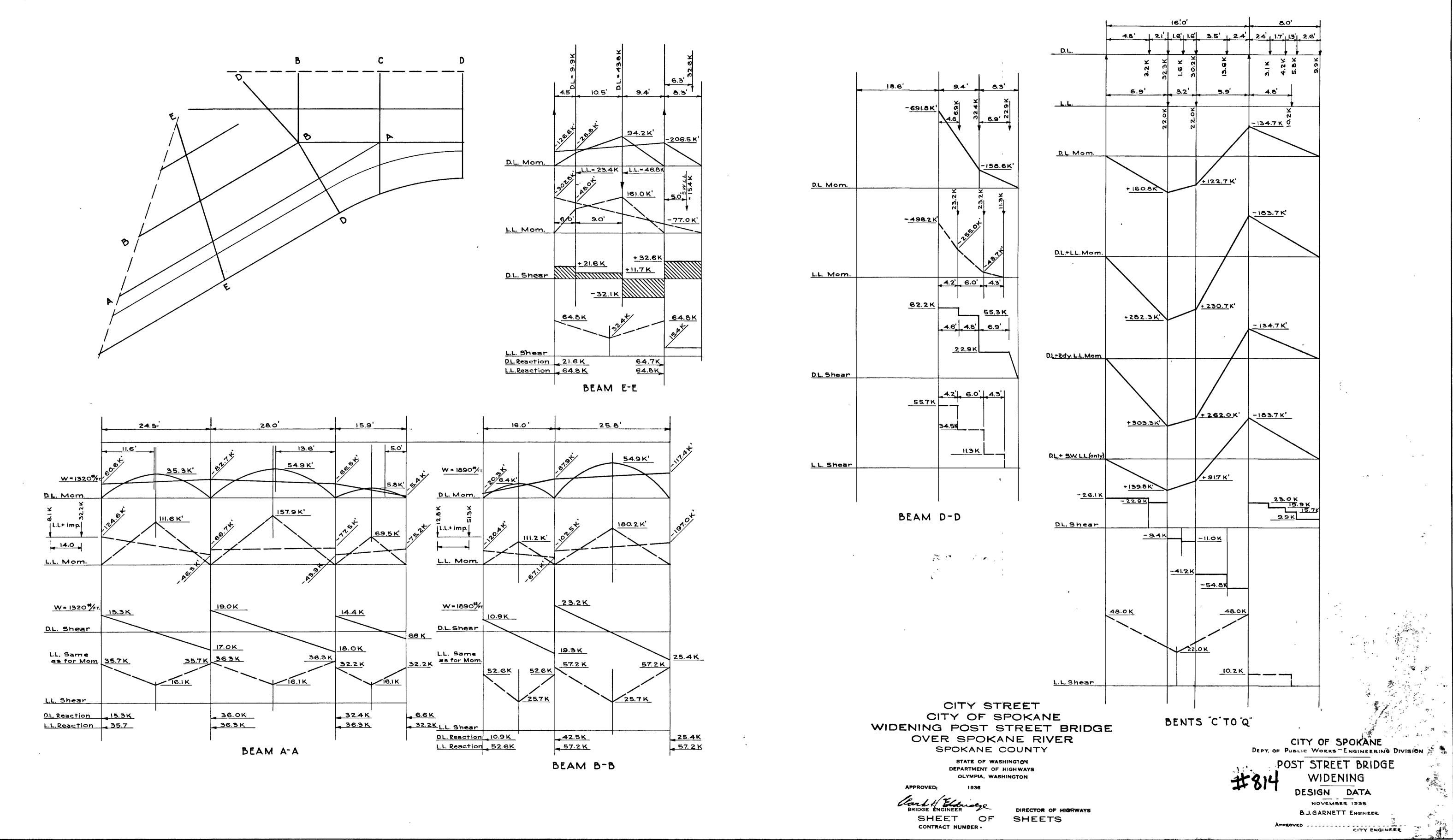
POST STREET BRIDGE

CONCRETE PLACING NOVEMBER 1935

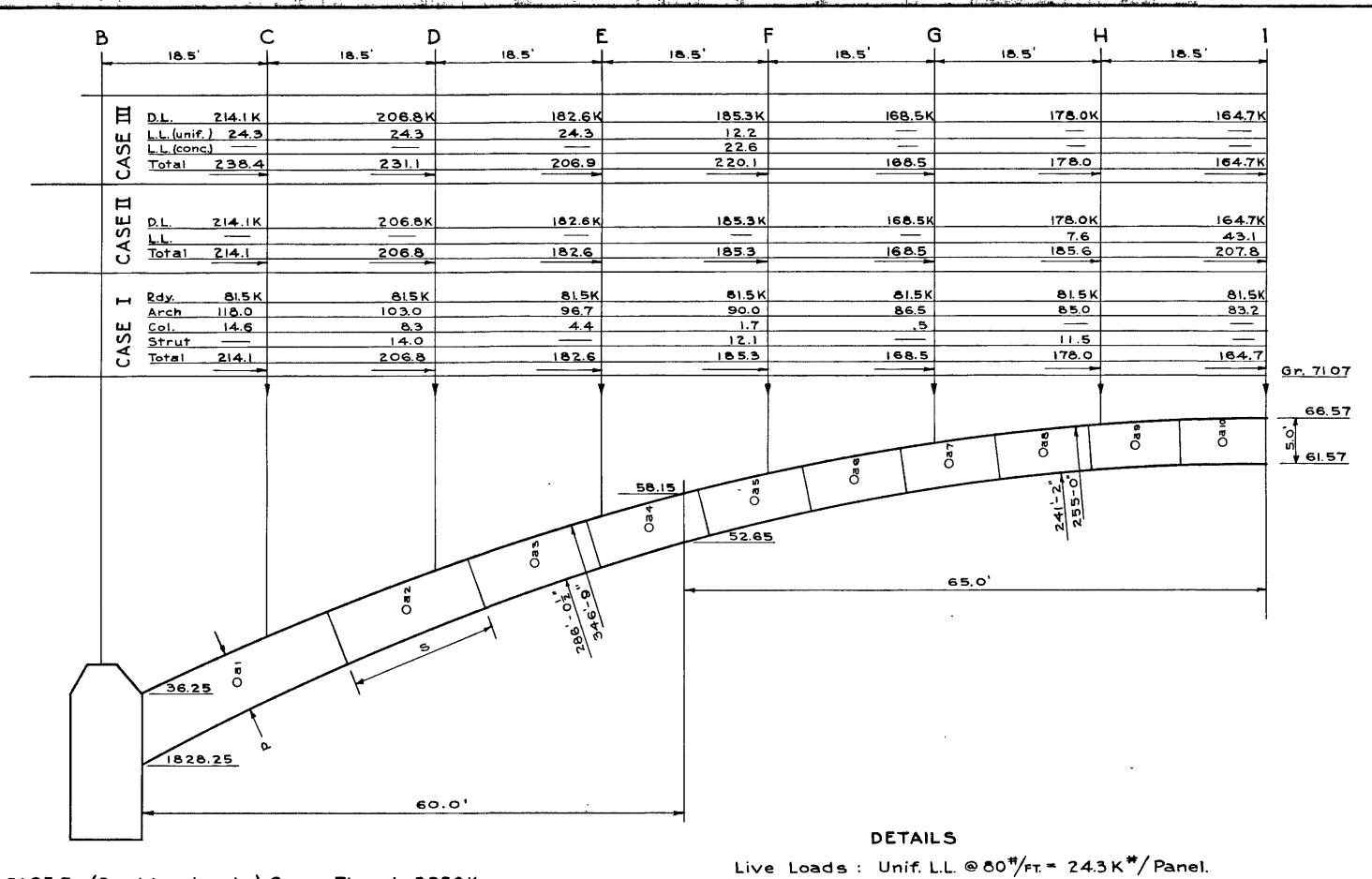
B.J. GARNETT, ENGINEER APPROVED ______CITY ENGINEER

8-20-36 See sheet 6-A for pouring of arch rib A.M.B. W.T. R Revision

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8/306



CASEI (Dead Load only) Crown Thrust = 2,350 K.

PT. THRUST HRUST ARM MOM. STRESS Ibs. sq. in. bd*/a' ac (ft.) H (ac) '# GM/bd2 Int. Ext.

ì	2640K	462	+ .19	+447 K	+ 72	+ 534	+390
2	2550	498	15	-352	- 99	399	597
3	2480	514	10	- 235	- 52	462	566
4	2430	526	08	-188	- 45	481	571
5	2430	538	06	-141	- 36	502	574
6	2390	540	+ .14	+339	+ 89	629	451
7	2370	540	+ .01	+ 23	+ 6	546	534
8	2370	543	+ .07	+164	+ 45	588	498
9	2350	543	04	- 94	- 26	517	569
		1	+ .01	+ 23	+ 7	550	536
10 CASI	2350 E II (De	543 ad Lo			<u> </u>		
	 				<u> </u>		
	 	ad Lo		on crow + 295 K	<u> </u>		οк.
CAS	EII (De	ad Lo	sd+LL.	on crow	n) C.T.	- 2,46	οк.
CAS	E II (De 2750K	ad Lo	+ .12	on crow + 295 K	n) C.T.	- 2,46 + 528	0 K. +432
CAS	EII (De 2750K 2660	480 520	+ .12 14	on crow + 295 K - 345	n) C.T. + 48 - 69	- 2,46 + 528 - 451	0 K. +432 589
1 2 3	EII (De 2750K 2660 2590	480 520 538	+ .12 14 06	n crow + 295 K - 345 - 148	n) C.T. + 48 - 69 - 33	+ 528 + 528 + 505	50 K. +432 589 571
CAS	2750K 2660 2590 2530	480 520 538 548	+ .12 14 06 + .03	+ 295 K - 345 - 148 + 74	n) C.T. + 48 - 69 - 33 + 18	+ 528 + 528 + 505 566	50 K. +432 589 571 530
2 3 4 5	EII (De 2750K 2660 2590 2530 2530	480 520 538 548 560	+ .12 14 06 + .03 + .05	+ 295 K - 345 - 148 + 74 + 123	n) C.T. + 48 - 69 - 33 + 18 + 31	+ 528 + 528 + 451 505 566 591	50 K. +432 589 571 530 529
CASI 1 2 3 4 5	EII (De 2750K 2660 2590 2530 2530 2500	480 520 538 548 560 563	+ .12 14 06 + .03 + .05 + .18	+ 295 K - 345 - 148 + 74 + 123 + 492 + 173	n) C.T. + 48 - 69 - 33 + 18 + 31 +130	+ 528 + 528 + 505 566 591 693	50 K. +432 589 571 530 529 433
CASI 1 2 3 4 5	EII (De 2750K 2660 2590 2530 2530 2500 2470	480 520 538 548 560 563	+ .12 14 06 + .03 + .05 + .18 + .07	+ 295 K - 345 - 148 + 74 + 123 + 492 + 173	n) C.T. + 48 - 69 - 33 + 18 + 31 +130 + 47	+ 528 + 528 + 505 566 591 693 610	50 K. +432 589 571 530 529 433

0	2770	501	T .O !	T 1/3	+ 40	0.0	3,3
9	2460	568	11	- 271	- 75	593	643
10	2460	569	14	- 345_	- 96	473	665
CAS	EIII (D	ead L	oad+Ll	L.on C-D	- E- F)C	::T.= 2,4	60 K
ļ	2790K	488	+ .52	+1525 K	+243	+731	+ 245
2	2690	526	21	- 516	- 102	424	628
3	2600	540	33	- 812	- 18i	359	721
4	2540	550	32	- 788	- 191	359	741
5	2540	562	34	- 836	- 202	360	764
6	2490	563	02	- 49	- 13	549	575
7	2470	563	+ .02	+ 49	+ 13	576	550
8	2470	567	+ .18	+ 443	+121	688	446
9	2460	567	+ .21	+ 517	+143	710	424
10	2460	568	+ .35	+ 860	+ 239	807	329

Conc. L.L. 18.0K × 16.0 × 10.0 × 1.13/9 × 16.0 = 22.6K/Panel (imp=13%). Arch Analysis:

Plus values of stress represent compression. tension, Plus values of ac show polygon below neutral axis. above " ".

Rib shortening taken as - 12° F.

														· · · · · · · · · · · · · · · · · · ·			
CAS	E	V	T	EN	1 P. – R	21B	SH.		ST	RE	55	=	6M	/b	d Z		
PT.	Aı	rm (ka)	M	= H+	(k	a)/°	_	40°	F	+	30) • F	R.	<u>s.(</u>	-12°)	
i	_	18.	.23	+	34	2	50	+	22	. 3	_	١	67	+	6	7	
2	_	10.	42	+	19	6	00	+	15	6	_	1	17	+	4	7	
3		5.	27	+	9	9	00	+	8	9	-		67	+	2	7	0
4	_	1.	34	+	2	5	00	+	2	5	-		19	+		8	
5	+	J.	73	1	3	2	00	-	3	3	+		25	_	ŧ	0	1
6	+	4.	14	_	7	8	00	_	8	.3	+		62	_	2	5	9
7	+	5.	91	-	11	1	00	_	12	0	+		90	_	3	6	
8	+	7.	14	_	13	4	00	-	14	8	+	١	1.1		4	4	:
9	+	7	96	-	14	9	00	-	16	6	+	2	.08	_	6	2	
10	+	8	.41	-	15	8	00	-	17	' පි	+	1	34	~	5	3	

			ARCH	DA	TA		p=6,-0	
PT.	S	ΣS	۵	_d	ď	ď	bd u"	bd ²
1	23.6	23.6	8.11	6.62	43.8	290,1	5720	445k
2	16.6	40.2	31.9	5.92	35.0	207.5	5110	363
3	13.8	54.0	47.1	5.58	31.1	173.7	4820	323
4	12.6	66.6	60.3	5.35	28.6	153.1	4620	297
5	11.6	78.2	72.4	5.23	27.4	143.1	4510	284
6	11.0	89.2	8 3.7	5.13	26.3	135.0	4430	273
7	10.5	99.7	944	5.08	25.8	131.1	4390	268
8	10.3	110.0	104.9	5.03	25.3	127.3	4350	263
9	10.1	120.1	115.0	5.01	25.1	125.8	4330	261
10	10.0	130.1	125.1	5.00	25. <i>0</i>	125.0	4320	259

SUMMARY OF STRESS Note-All stresses comp. exc. as noted.

	INTRADOS								EXTRADOS							
PT.	D. L. (only) I	D.L.+L.L on crown II	D.L.+L.L. C-0-E-F	TEM	P R.S.	IV	MAX.	MIN.	D.L. (only) I	Crown C-D-E-F 108 D. C. MAX. MIN						
				-40°	+30°	R.S.				II	II III	-40°	+30°	R.S.	MAX.	MIN.
1	534	528	731	223	- 167	67	1021	361	390	432	245	- 223	167	- 67	599	- 45
2	399	451	424	156	- 117	47	654	274	597	589	628	- 156	117	- 47	745	38 <i>0</i>
3	462	505	359	89	- 67	27	621	292	566	571	721	- 89	67	- 27	788	450
4	481	566	359	2.5	- 19	8	599	340	571	530	741	- 25	19	- 8	760	497
5	502	591	360	- 33	25	- 10	616	317	574	529	764	33	- 25	10	807	504
6	629	693	549	- 83	62	- 25	755	441	451	433	575	83	- 62	25	683	371
7	546	610	576	- 120	90	- 36	700	390	534	516	550	120	- 90	36	706	426
8	588	615	688	- 148	[11	- 44	799	396	498	519	446	148	- 111	44	711	335
9	517	593	710	- 166	208	- 62	918	289	569	643	424	166	- 208	62	871	216
10	550	473	807	- 178	134	- 53	941	242	536	665	329	178	- 134	53	896	195

CITY STREET CITY OF SPOKANE WIDENING POST STREET BRIDGE OVER SPOKANE RIVER SPOKANE COUNTY

> STATE OF WASHINGTON DEPARTMENT OF HIGHWAYS OLYMPIÄ, WASHINGTON

CONTRACT NUMBER -

SHEETS

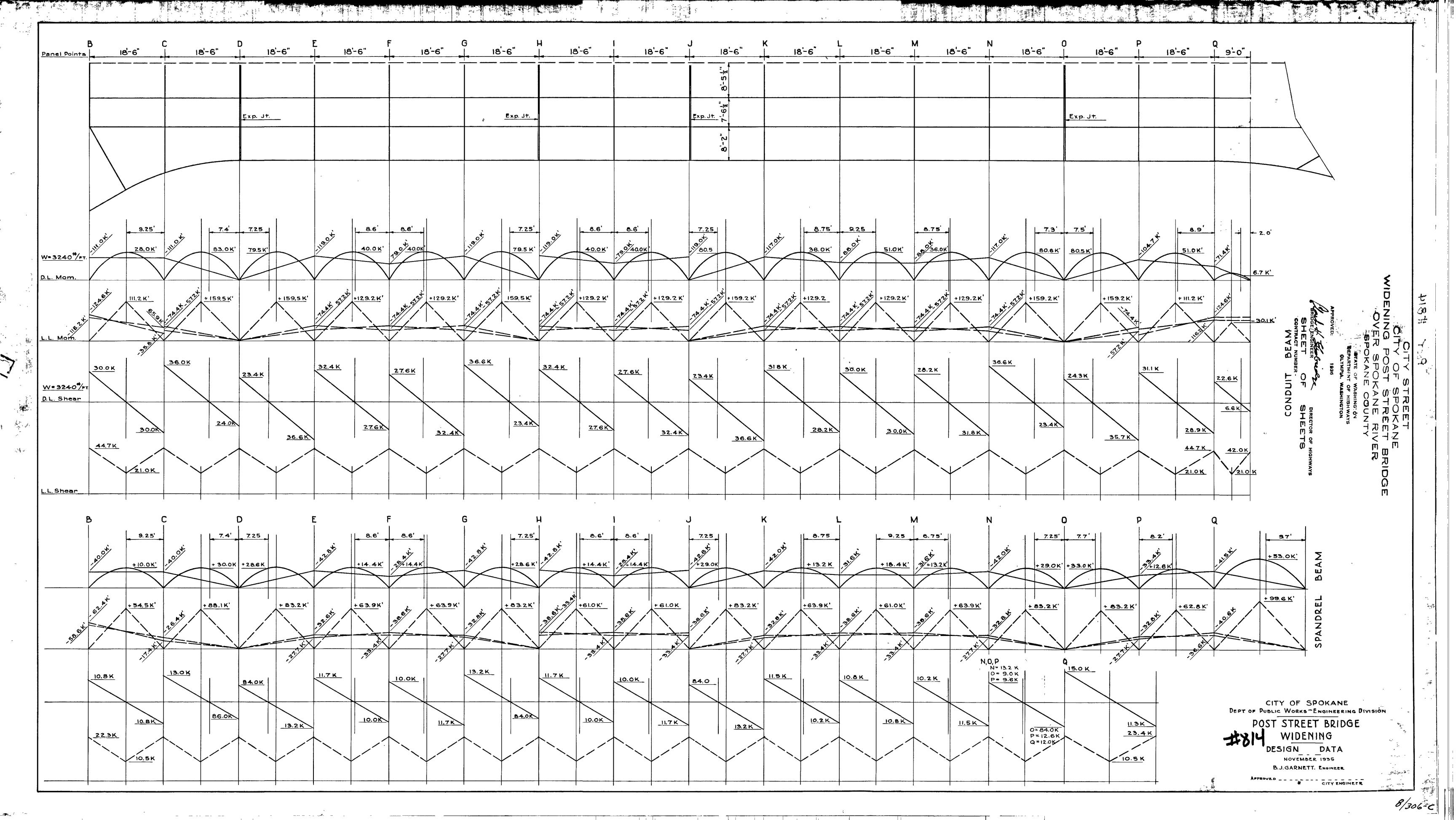
CITY OF SPOKANE DEPT. OF PUBLIC WORKS ENGINEERING DIVISION

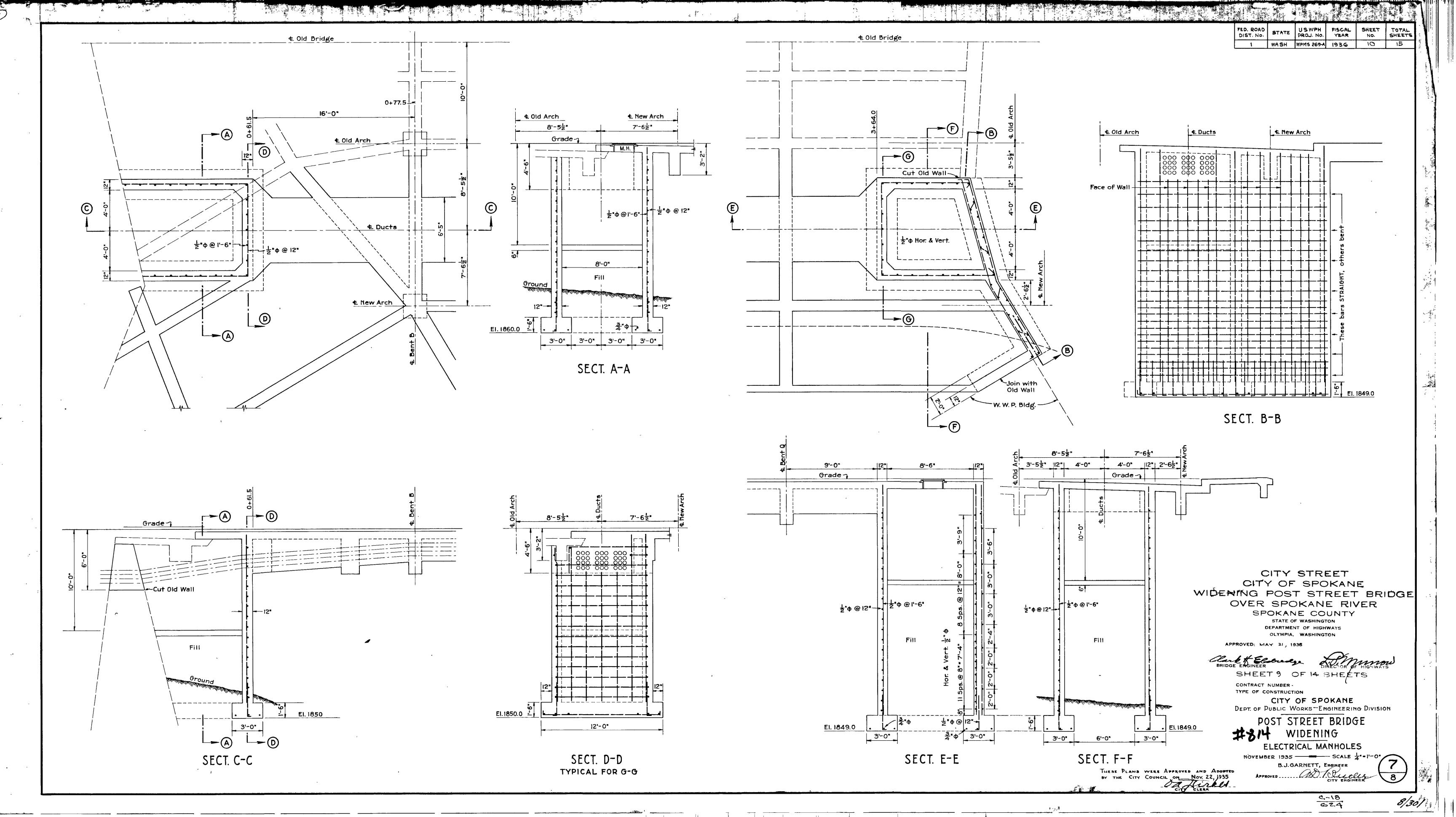
DESIGN DATA NOVEMBER 1935

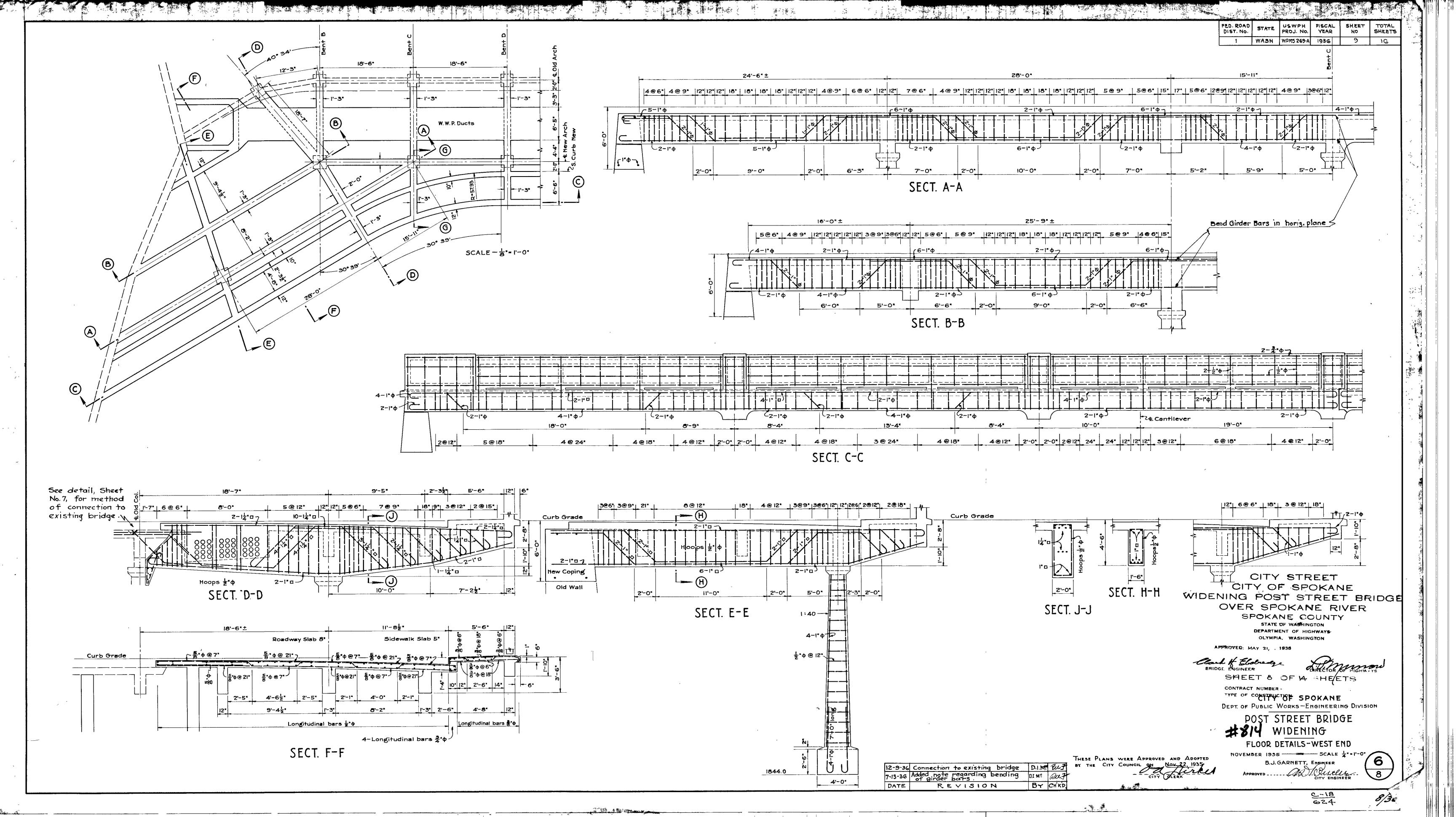
B.J.GARNETT ENGINEER

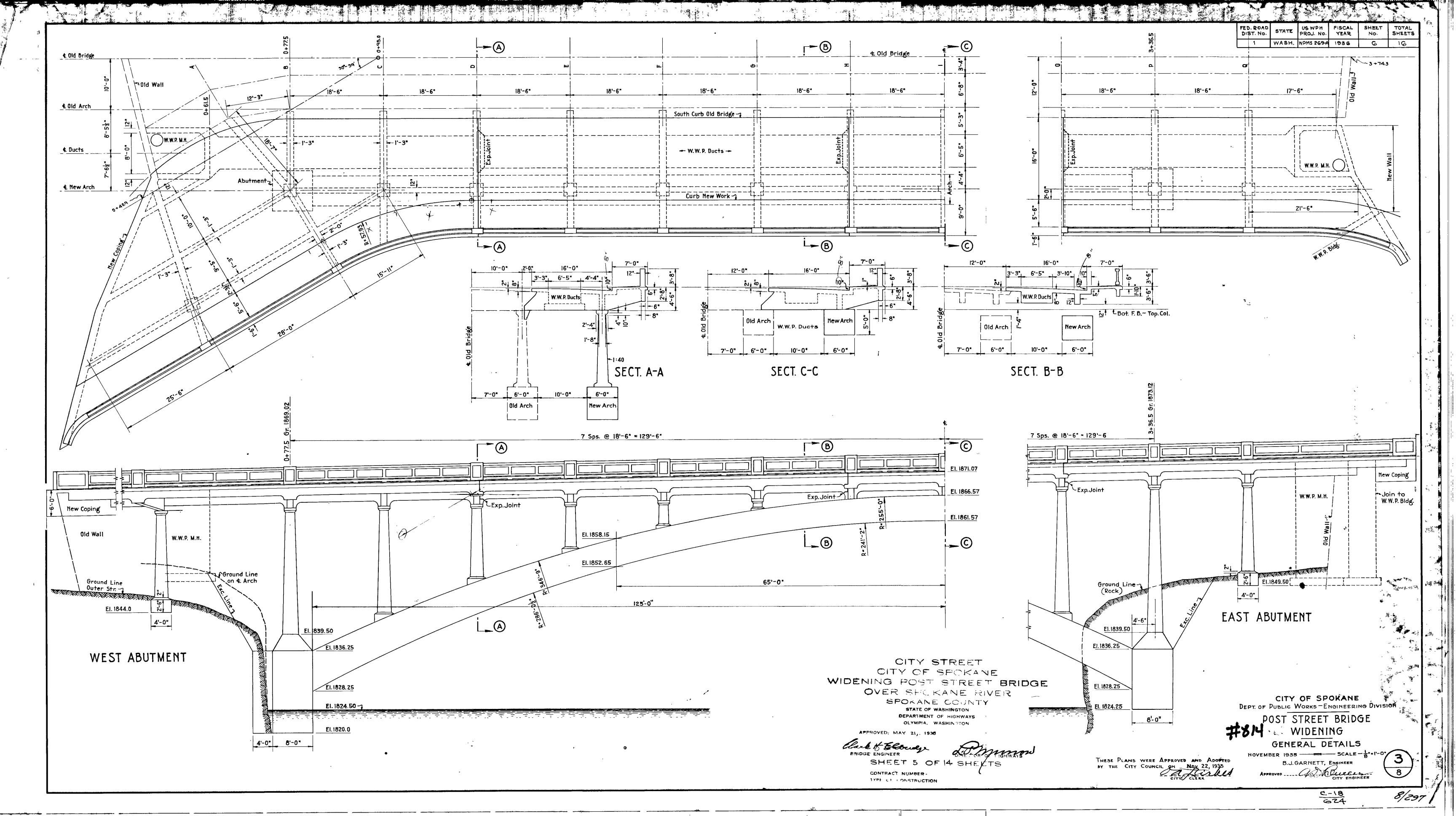
EPPROVED ______CITY ENGINEER

8/306-A

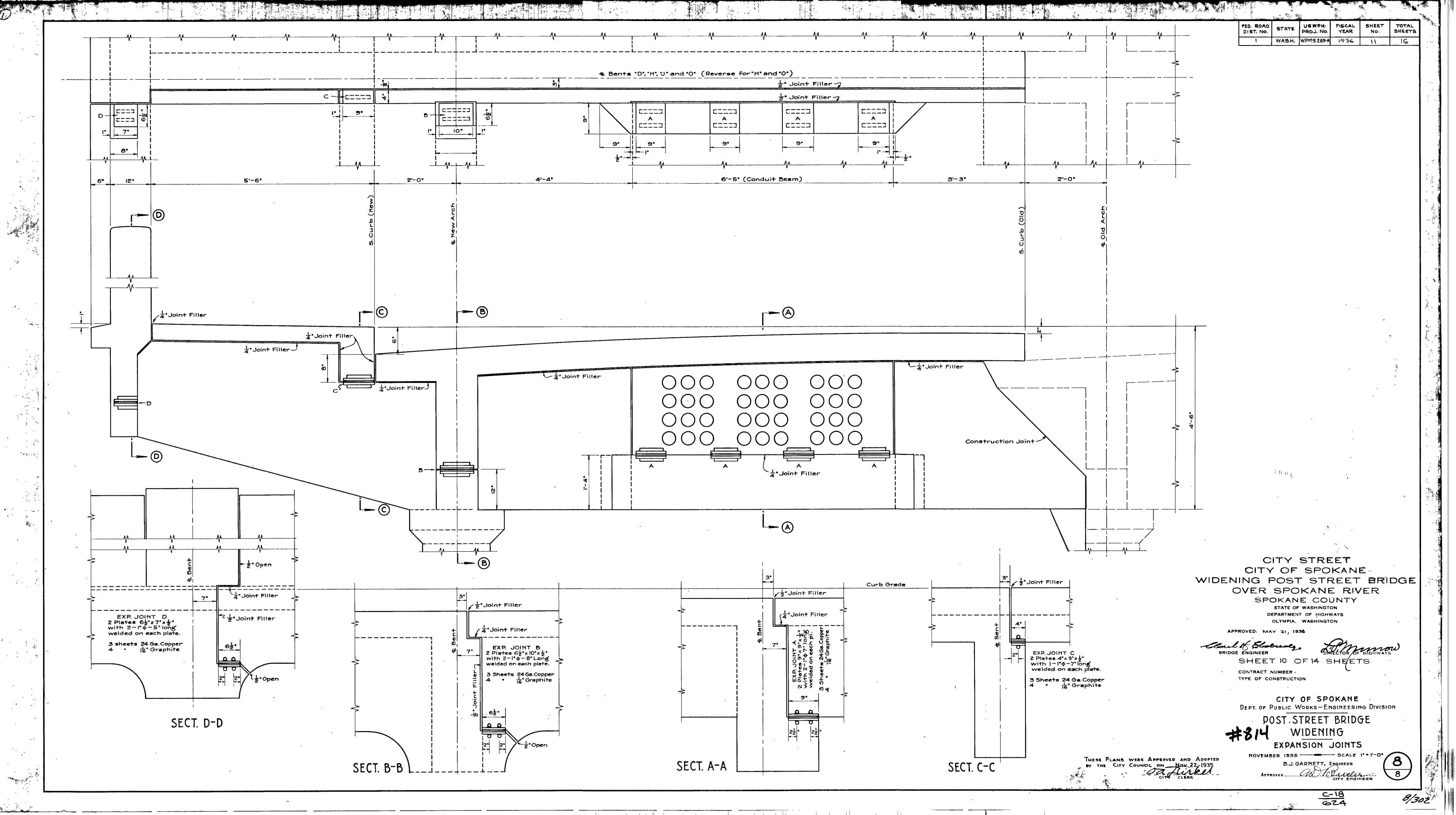






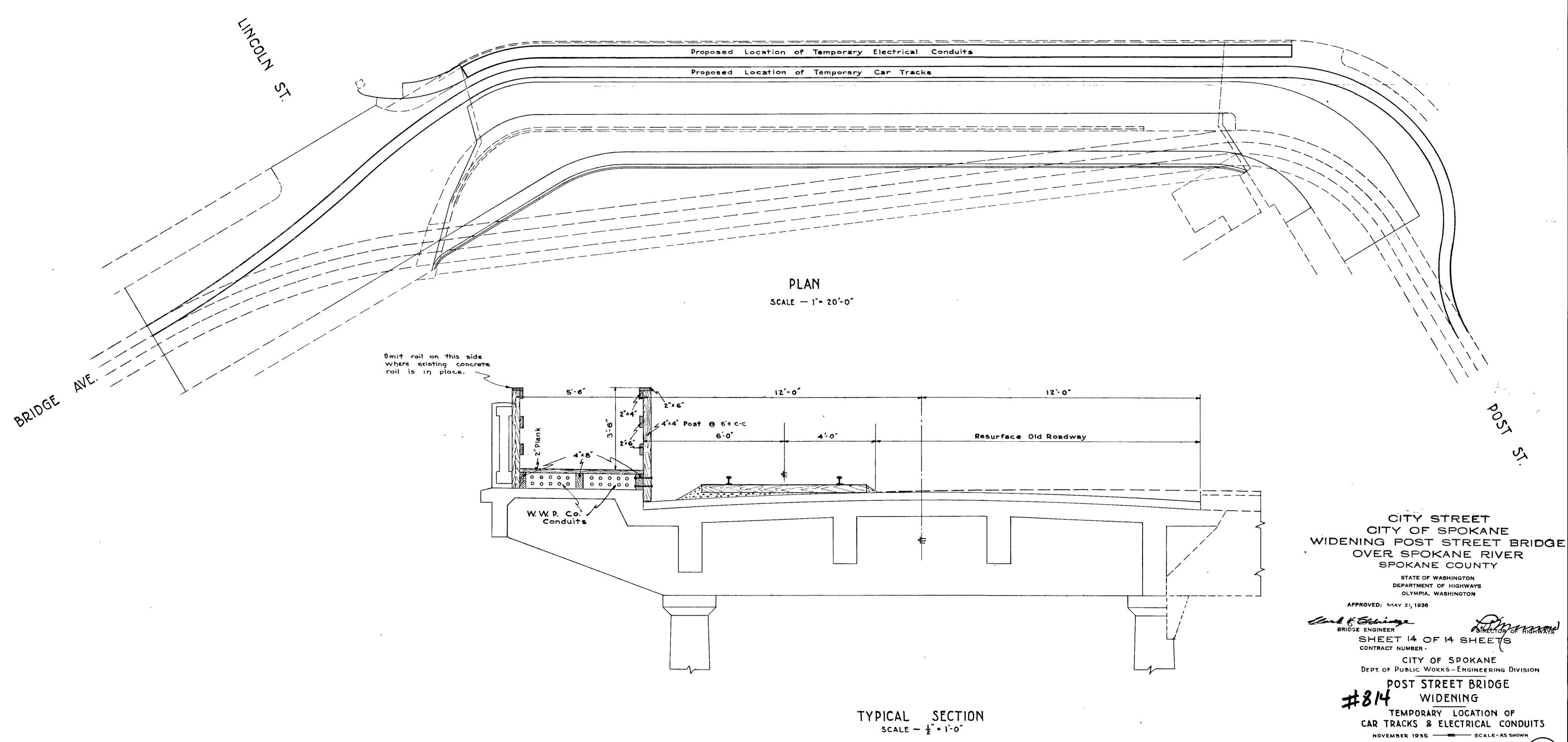


RESURVEY & EXTENSION OF POST'S ADDITION Old West Wall -75 TOPOGRAPHY SCALE - 1" = 20'-0" Vertical Cliff General Notes: All materials and workmanship shall conform to the requirements ______ of the Standard Specifications of the State of Washington Department of Highways dated April 1935. All Concrete in arch abutments shall be of Class B mix with 3" maximum size of coarse aggregate All other concrete shall be of Class A mix with 3" maximum size of coarse aggregate except that in sidewalk slabs and handrailings the maximum size of coarse aggregate shall be 11/2. RESURVEY & ADDITION TO SPORANE FALLS SOLID ROCK WEST EAST ABUTMENT ABUTMENT EL.1824.50 HIGH WATER LINE 4-15-35 -GROUND LINE AT & OF NEW ARCH CITY STREET PROFILE APPROXIMATE QUANTITIES CITY OF SPOKANE WIDENING POST STREET BRIDGE SCALE - 1" = 20'-0" 172 Cu.Yds. -Structure Excavation OVER SPCKANE RIVER RESURVEY & EXTENSION 300 " " -Special Embankment OF POST'S ADDITION - Concrete Class "A" in place 975 " " SPOKANE COUNTY 95 STATE OF WASHINGTON 15 DEPARTMENT OF HIGHWAYS 107,000 Lbs. - Steel Reinforcing Bars in place OLYMPIA. WASHINGTON Expansion Joints Lump Sum CITY OF SPOKANE APPROVED MAY ZI, 1936 -Temporary Wood Walk and Railing H M.B.M. DEPT. OF PUBLIC WORKS-ENGINEERING DIVISION Clark & Elbridge ... Removing Existing Steel Bridge Lump Sum LOCATION POST STREET BRIDGE Removing portion of Concrete Wall SHEET 3 OF 4 SHEETS WIDENING SCALE - 1" = 50'-0" portions -CONTRACT NUMBER --Asphaltic Concrete Pavement Type I-1, Class E 177 Tons LOCATION TYPE OF CONSTRUCTION " I-1 " F 40 " THESE PLANS WERE APPROVED AND ADOPTED B.J.GARNETT, ENGINEER BY THE CITY COUNCIL ON Nov. 22, 1935 624



DISTINO. STATE PROJ. NO. YEAR NO. SHEETS

1 WASH. WPMS-269-1 1936 15 16



C-18

8/306

